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Exam to the Lecture Traffic Dynamics and Simulation SS 2021

Total 120 points

Problem 1 (40 points)

Given is following acceleration equation for a car-following model as a function of the gap s , the speed v , and the leader's speed v_l :

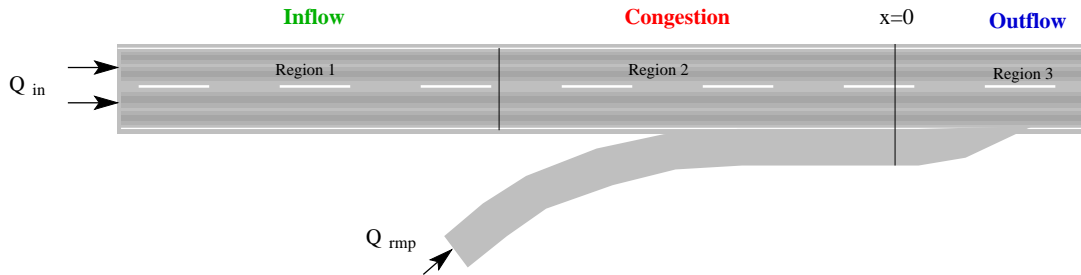
$$\frac{dV}{dt} = \min \left\{ a \left[1 - \left(\frac{v}{v_0} \right)^4 \right], a \left[1 - \left(\frac{s}{s^*} \right)^4 \right] \right\}, \quad s^* = s_0 + vT + \frac{v(v - v_l)}{\sqrt{2ab}}$$

- (a) Is this a time-continuous or a time-discrete car-following model?
- (b) Discuss, in a few words, the first term $a(1 - (v/v_0))^4$ and give the meaning of the model parameters a and v_0 .
- (c) Discuss the second term $a(1 - (s/s^*)^4)$.
- (d) When deriving the fundamental diagram (homogeneous steady-state relation), one has to set $\frac{dV}{dt} = 0$ and $v_l = v$. Why? Discuss by referring to the definition of the fundamental diagram.
- (e) Give the steady-state speed for sufficiently large gaps where no interaction occurs.
- (f) Show that, in the steady state, the transition from free traffic to the congested state is abrupt and at a gap $s_0 + v_0T$. Discuss the model parameters s_0 and T .
- (g) Give the steady-state gap as a function of the speed for speeds less than the desired speed.
- (h) Given is a traffic flow of identical drivers and vehicles of 5 m length. Give the maximum density for $s_0 = 3$ m.
- (i) Give the fundamental diagram $Q_e(\rho)$ of this model. Assume a free-flow speed of v_0 and a steady-state gap $s_e(v) = s_0 + vT$ for speeds $v < v_0$.
- (i) A driver driving according to this model approaches a red traffic light at the free-flow speed. At which distance from the traffic light this driver begins to brake? Give the general result as a function of s_0 , v_0 , T , and b and identify this distance as the sum of the minimum gap, the distance covered during the reaction time, and the braking distance for a constant deceleration and associate the reaction time and the braking deceleration with model parameters.

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Problem 2 (40 points)

Given is a freeway road section with an on-ramp merging to the main road effectively at the location $x = 0$ according to following sketch (no jammed region 2 at the beginning):



For the analysis, assume a macroscopic LWR model and a triangular fundamental diagram with the parameters

$$V_0 = 25 \text{ m/s}, \quad T = 1.6 \text{ s}, \quad \rho_{\max} = \frac{1}{l_{\text{eff}}} = 0.1 \text{ m}^{-1}.$$

- (a) Assuming a ramp of merging length l_{rmp} , the continuity equation of this situation for the total density and flow (summed over both lanes) can be written as

$$\frac{d\rho^{\text{tot}}}{dt} + \frac{dQ^{\text{tot}}}{dx} = \begin{cases} \frac{Q_{\text{rmp}}}{L_{\text{rmp}}} & x \in [-L_{\text{rmp}}, 0], \\ 0 & \text{otherwise.} \end{cases}$$

Calculate for stationary situations ($\frac{d}{dt} = 0$ but of course $\frac{d}{dx} \neq 0$ in the merging region) the total main flow $Q^{\text{tot}}(x)$ as a function of the constant total main inflow Q^{in} and the ramp flow Q_{rmp} .

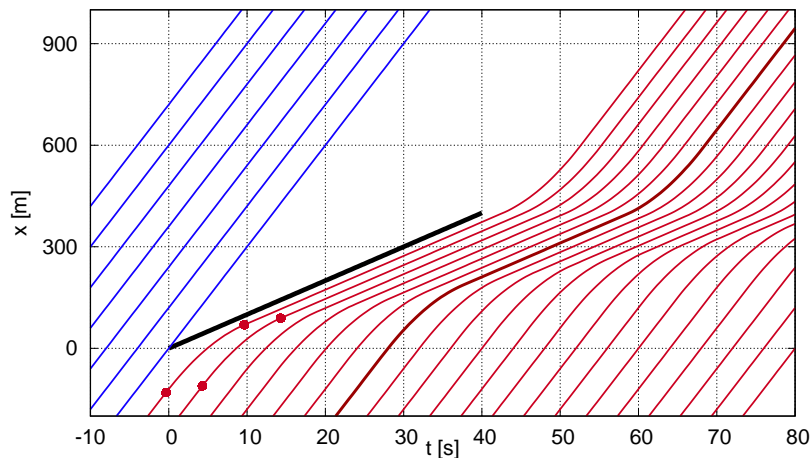
- (b) Calculate the numerical values for the critical density ρ_c per lane at the free-congested transition, the maximum flow per lane, and the main-road capacity.
- (c) Draw the fundamental diagram for the total flows on the main road
- (d) Calculate the propagation velocity of small perturbations in free flow and in congested traffic.
- (e) The ramp flow is constant $Q_{\text{rmp}} = 400 \text{ veh/h}$ and, at the beginning of the analysis, the main inflow $Q_{\text{in}} = 3000 \text{ veh/h}$. Argue that these demands will not lead to a traffic breakdown (watch out for the units!). Draw the total flows and densities of Region 1 (upstream of the ramp) and Region 3 (downstream) into the fundamental diagram of Question (c).
- (f) At 16:00h, the main inflow as observed by a stationary detector 6 km upstream ($x = -6 \text{ km}$) suddenly increases from 3000 veh/h to 4200 veh/h while the ramp flow remains constant at 400 veh/h. Argue that this increase of the demand will lead to a breakdown. Determine the location and time of the breakdown. *Hints:* Here, you can assume a negligible ramp length. Watch out for the finite propagation velocity of flow changes in free flow.

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- (g) Calculate the propagation velocity of the upstream front of the congestion resulting from the breakdown. Draw the traffic states of the Regions 1, 2, and 3 into the fundamental diagram of Question (c).
- (h) At 17:00 h, the traffic flow immediately upstream of the transition free \rightarrow jammed drops from 4 200 veh/h to 2 200 veh/h. Calculate the maximum length of the congestion and the loss of time for the most unhappy driver (arriving just at 17:00 h at the rear end of the congestion).
- (i) Calculate the time at which the congestion dissolves assuming no further demand changes after 17:00. Motivate why the dissolution takes a longer time than the buildup although the excess supply after 17:00 is equal to the excess demand before.

Problem 3 (25 points)

Given is following trajectory diagram for one lane of a freeway:

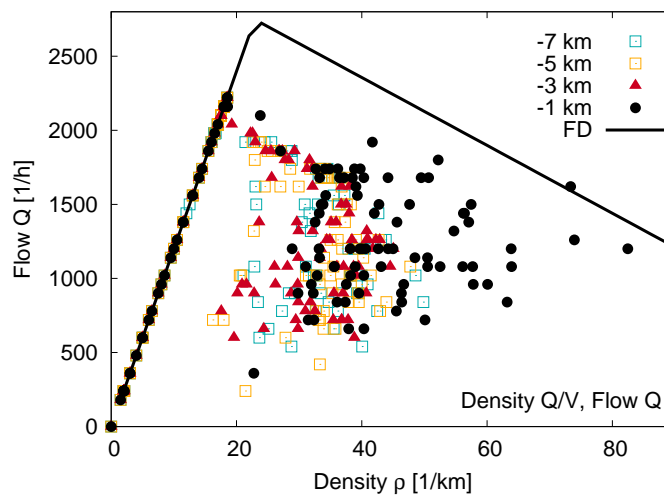


- (a) Which situation can lead to such trajectories? (Notice that the thick black line represents a vehicle as well).
- (b) Determine density, speed, and flow for the three regions
- upstream, $x \leq 0$
 - congested region
 - outflow state ($t > 60$ s and $x > 600$ m).
- (c) Determine the propagation velocities of the upstream and downstream transition zones of the congestion.
- (d) Estimate the braking deceleration (the braking phase is between the two red closed circles).

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Problem 4 (15 points)

Given is a flow-density scatter plot obtained from virtual stationary detectors of a microscopic traffic flow simulation (symbols) together with the fundamental diagram of the simulated model.



- (a) Obviously, the points of the scatter plot do not lie on the fundamental diagram although a deterministic micromodel with identical drivers and vehicles has been simulated. What are possible reasons?
- (b) Discuss further irregularities or stochastic elements of real traffic that may also lead to the observed scattering.